

University of Notre Dame  
Department of Aerospace and Mechanical Engineering

AME 20241-02

Mechanics of Solids

Spring 2008

**EXAM #1**

February 19, 2008

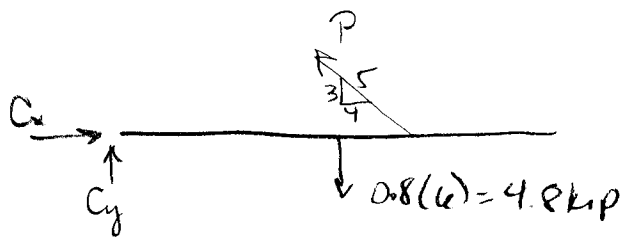
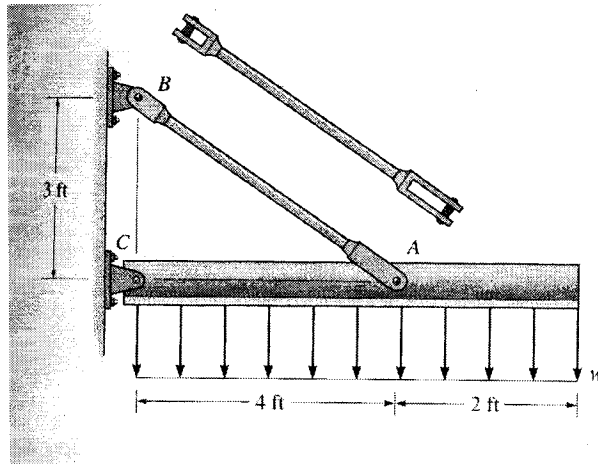
Name: Solutions

Question	Points	Score
1	25	
2	<del>25</del> 20	
3	<del>25</del> 30	
4	25	
total	100	

1. (25 pts.) The hanger assembly below is used to support a distributed loading of  $w = 0.8 \text{ kip/ft}$ .

a) Determine the average shear stress in the 0.4 in. diameter bolt at A and the average tensile stress in rod AB, which has a diameter of 0.5 in.

b) If the yield shear stress for the bolt is  $\tau_y = 25 \text{ ksi}$ , and the yield tensile stress for the rod is  $\sigma_y = 38 \text{ ksi}$ , determine the factor of safety with respect to yielding in each case.

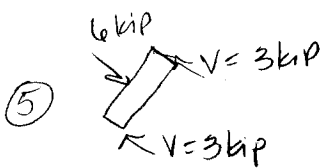


$$\sum M_C = 0 = -4.8 \text{ kip}(3 \text{ ft}) + \frac{3}{5} P(4 \text{ ft})$$

$$P = 6 \text{ kip} \quad (5)$$

For bolt A:

$$\tau = \frac{V}{A} = \frac{3}{\frac{\pi}{4}(0.4)^2} = 23.9 \text{ ksi} \quad (5)$$



$$FS: \frac{\tau_y}{\tau} = \frac{25}{23.9} = 1.05$$

For rod AB

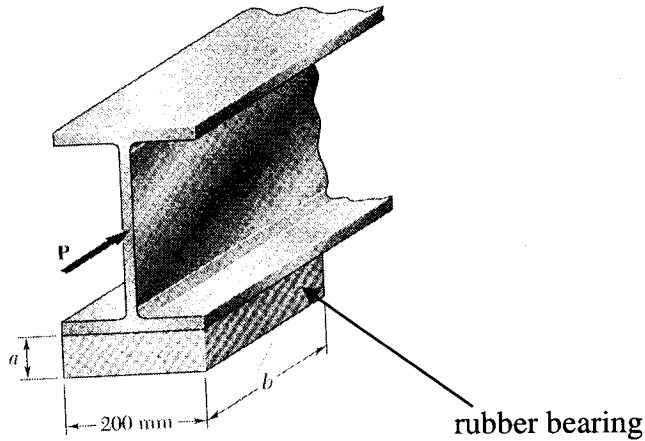
$$\sigma = \frac{P}{A} = \frac{6}{\frac{\pi}{4}(0.5)^2} = 30.6 \text{ ksi} \quad (5)$$

$$FS \quad (5)$$

$$FS = \frac{\sigma_y}{\sigma} = \frac{38}{30.6} = 1.24$$

2. <sup>20</sup> (25 pts.) A rubber bearing ( $G = 0.9 \text{ MPa}$ ) is used to support a bridge girder as shown to provide flexibility during earthquakes. The beam must not displace more than 10 mm when a 22 kN load is applied as shown. Knowing that the maximum allowable shear stress is 420 kPa, determine:

- a) the smallest allowable dimension  $b$  to the nearest mm
- b) the largest required thickness  $a$  " " " "



⑩ a)  $\tau_{ave} = \frac{V}{b \cdot w} \Rightarrow 410 \times 10^3 \text{ Pa} = \frac{22 \times 10^3 \text{ N}}{b \cdot (0.2 \text{ m})}$   $\Rightarrow b = 268.3 \text{ mm}$   
have to round up to keep  $\tau < \tau_{allow}$   $b = \underline{269 \text{ mm}}$

⑩ b)  $\tau = G\gamma \Rightarrow \gamma = \frac{\tau}{G} = \frac{410 \times 10^3}{0.9 \times 10^6} = .455$



$\delta \approx \tan \gamma = \frac{10 \text{ mm}}{a} = .455$   
(assumes small angle)

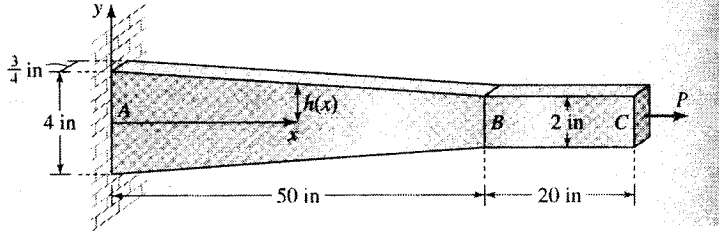
or  $\tan \gamma = \frac{10 \text{ mm}}{a} = .489$

$a = 21.98$   
have to round down to keep  $\delta < 10 \text{ mm}$   $a = \underline{21 \text{ mm}}$

$a = 20.44$   
have to round down to keep  $\delta < 10 \text{ mm}$   $a = \underline{20 \text{ mm}}$

3. (25 pts.) A rectangular aluminum bar ( $E_{al} = 10,000 \text{ ksi}$ ,  $\nu = 0.25$ ) of a  $\frac{3}{4}$  in. thickness consists of uniform and tapered cross sections, as shown. The depth of the tapered section varies as  $h(x) = 2 - 0.02x$ . Determine:

- a) The elongation of the bar under a load  $P = 10 \text{ kips}$ .
- b) The change in dimension in the y direction in section BC.



⑩ a)  $\delta = \delta_{AB} + \delta_{BC}$  ②

$$\delta_{BC} = \frac{PL}{EA} = \frac{(10 \times 10^3 \text{ lb})(20 \text{ in})}{(10,000 \times 10^3 \text{ lb/in}^2)(\frac{3}{4} \text{ in})(2 \text{ in})} = .0133 \text{ in} \quad ③$$

$$\delta_{AB} = \int_{x=0}^{x=50} \frac{P}{EA(x)} dx \quad ④$$

where  $A(x) = 2h(x)$

$t = \frac{3}{4} \text{ in}$

$h(x) = 2 - 0.02x$

$$\delta_{AB} = \frac{P}{2Et} \int_{x=0}^{x=50} \frac{dx}{(2 - 0.02x)} = \frac{10 \times 10^3}{(2)(10,000 \times 10^3)(\frac{3}{4} \text{ in})(-0.02)} \ln(2 - 0.02x) \Big|_0^{50}$$

$$= \frac{10}{(2)(10,000)(\frac{3}{4})(-0.02)} [\ln(1) - \ln(2)] = .0231 \text{ in} \quad ⑤$$

$\delta = .0133 \text{ in} + .0231 \text{ in} = \underline{0.036 \text{ in}}$  elongation

⑩ b)  $\epsilon_x = \frac{\delta_x}{L_x}$  in section BC

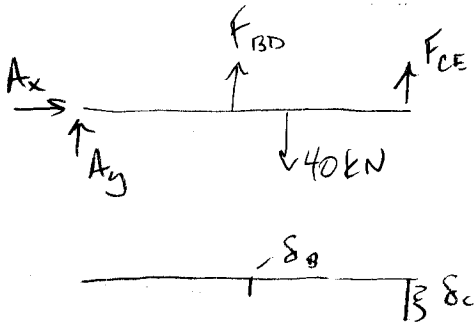
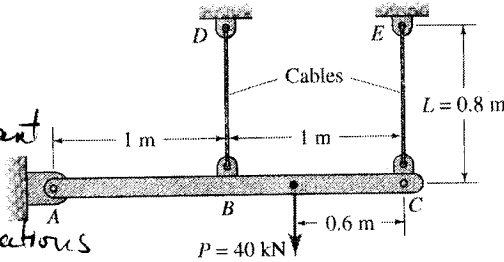
$$\epsilon_x = \frac{.0133}{20 \text{ in}} = 655 \mu\text{in/in}$$

$$\epsilon_y = -\nu \epsilon_x = -(0.25)(655 \mu\text{in/in}) = -164 \mu\text{in/in}$$

$$\delta_y = \epsilon_y L_y = (-164 \frac{\mu\text{in}}{\text{in}})(2 \text{ in}) = \underline{-333 \mu\text{in}}$$
 contraction

4. (25 pts.) A rigid horizontal bar ABC is supported by a hinge at A by by two steel cables BD and CD, which are of equal length,  $L = 0.8$  m, and cross-sectional area  $A = 140$  mm<sup>2</sup>. Find the stress in each cable due to a vertical force of  $P = 40$  kN.

Structure is statically indeterminate  
 4 unknowns  
 3 equilibrium equations



Equilibrium  
 $\sum M_A = 0: F_{BD} + 2F_{CE} - 40(1.4) = 0$  (1)

Compatibility

$2\delta_B = \delta_C$  by similar triangles (5)

Force-Displacement  
 $\delta = \frac{PL}{EA}$  (5)

$2\delta_B = \delta_C \Rightarrow \frac{2F_{BD}L}{EA} = \frac{F_{CE}L}{EA} \Rightarrow 2F_{BD} = F_{CE}$  (2)

Solve eqns (1) + (2) to get  $F_{BD} = 11.2$  kN,  $F_{CE} = 22.4$  kN (5)

$\sigma_{BD} = \frac{11.2 \times 10^3}{140 \times 10^{-6}} = 80$  MPa

$\sigma_{CE} = 160$  MPa (5)